

Recent Research in Seismic-Resilient Wood Buildings

Asif IqbalJohn van de LindtShiling PeiThang DaoPouria BahmaniAndre BarbosaMarjan Popovski





Research Projects:

- Seismic Risk Reduction of Soft-Story Buildings (NEES-Soft)
- Seismic Resilient Tall Wood Buildings (NHERI Tall Wood)
- CLT-Light Frame System Hybrid Buildings (CLT-LiFS)
- Performance Factor (R-Factor) for CLT Shear Wall Systems
- Post-tensioned Timber (Pres-Lam) Systems



NEES-Soft: CLT Retrofit





Seismic Evaluation and Retrofit of Multi-Unit Wood-Frame Buildings With Weak First Stories

FEMA P-807 / May 2012







NEES-Soft: PBSR



NEES-Soft: Retrofitted (PBSR) and Un-retrofitted Comparison

NEW ZEALAND SOCIETY FOR EARTHQUAKE ENGINEERING





NEES-Soft: Collapse Test







NEW ZEALAND SOCIETY FOR EARTHQUAKE ENGINEERING 17th U.S.-Japan-New Zealand Workshop on the Improvement of Structural Engineering and Resilience NHERI T&LLWOOD PROJECT

- **NHERI Objective: Develop and validate a Resilience-based** seismic design methodology for tall wood buildings
- Website: nheritallwood.mines.edu





Shiling Pei

COLORADOSCHOOLOFMINES.



UNIVERSITY of

WASHINGTON



Keri Ryan





LEHIGH

Richard Sause





WASHINGTON STATE

🕵 UJNIVERSITY



John van de Lindt Colorado





Thomas Robinson

LEVER ARCHITECTURE



Eric McDonnell

kpff











Marian Popovski













Shake-table Diaphragm Testing

- Two Diaphragm Designs for all Three Wall Systems
 - Roof CLT Panels + Concrete Topping (Composite slab)
 - Floor level 3-ply CLT Panels









Shake-table Diaphragm Testing

Average accelerations at diaphragm levels



Northridge (MCE) – Scale Factor 1.2



Uniform accelerations at the roof level



Shake-table Diaphragm Testing

Accelerations for Northridge (MCE) (Scale Factor 1.2 : Avg. Peak Floor Accel. (g) = 0.873)







CLT-LiFS wall test



Steel beam

CLT-LiFS Walls



STIFFNESS MATRIX OF 4-NODE ROCKING ELEMENT

Nodal displacements of 2-node element can be expressed by that of 4-node element, via ^{Noz} a transformation matrix $[B_2]$ (Node 5 and 6 are virtual nodes not included in stiffness matrix):

 $[U_{2-node}^{o}] = [B_2][U_{4-node,8-DOF}^{local}]$ Assume that $\Delta y_1 = \Delta y_4 = \Delta y_5 = \Delta y_6 = u_{io}$ and $\Delta y_2 = \Delta y_3 = u_{jo}$. The nodal displacements of 4-node element can also be expressed by that of 2-node element, via matrix $[B_3]$:

 $\left[U^{local}_{4-node,8-DOF}\right] = [B_3][U^o_{2-node}]$



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MOISTURE DIFFUSION AND CREEP MODELS



Moisture content tests

- Three types of specimen.
- Three environmental conditions.

Calculate moisture content

 $MC = \frac{W_1 - W_0}{W_0} \times 100 \text{ percent}$

Obtaining diffusion coefficients



6.6×12×12 in. Specimen weight over time

3.55%, MG2 =17.35%, MC3 =13.55% Time, days

• Various specimen types.

• Different loading levels.

Creep tests: Measuring deformations

Multiple locations. •

Lognormal Distribution μ =1.4, σ =20.

150 200

μ_k (10¹² kPa.min) for 5 layers 15%

100

0.9

0.8

0.7

0.6

0.5

0.4

0.2 0.1

J. 0.3 • Using DEMEC and gauges.

Numerical: Using the four-element model.



Date

250 300 350 13.5

13

12.5

11

% 12 0 11.5





Fig. 15. Axial strain history for different CLT configurations (1 in. = 2.54 cm); (a) moisture content profile; and (b) axial strain







CLT-LiFS Building test

Realtime Hybrid Simulation of CLT-LiFS Building (Under preparation)

CLT-LiFS Building







Performance (R) Factors for CLT Shear Wall Systems

- For ASCE 7-16 ELF Procedure
- Follows FEMA P-695 methodology
- Involves development of design method
- Archetypes developed for shear walls testing
- Includes uncertainties







Performance (R) Factors for CLT Shear Wall Systems

- Different wall configurations tested
- Boundary conditions and gravity load considered
- Nonlinear Time History analysis performed
- Maximum inter-story drift recorded
- Cumulative Density Function (CDF)s plotted



200





Post-tensioned Timber Systems

- Full scale model
- CLT walls and Glulam frames
- CLT Floor added
- Practical connection details
- Walls testing complete









Post-tensioned Timber Systems

- Single and Coupled Walls tested
- Expected performance
- Almost no damage
- Numerical models developed
- Key parameters identified







Acknowledgements













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