Erratum (November 25, 2019)

FEMA P-1100 Report, Vulnerability-Based Seismic Assessment and Retrofit of One- and Two-Family Dwellings, October 2019 edition

After the FEMA P-1100 Report was printed, errors were discovered in tables in Figures 5.4-9, 5.4-11, and 5.4-15. The corrected tables are presented below.

EARTHQUAKE RETROFIT SCHEDULE (S _{DS} = 1.0) Two Sections of Wall at Front of Garage - Only																
WEIGHT CLASSIFICATION	2	© Mark row that applies ©	MINIMUM TOTAL REQUIRED LENGTH OF EACH SECTION OF WOOD STRUCTURAL PANEL SHEAR WALLS For a Two Section of Wall Option						FOUNDATION SILL ANCHORAGE Min. No. of Foundation Connectors or Anchors at				G FLOOR FRAMING TO WALL CONNECTION Min. No. of Connectors			
			Length per Section for a Two Section Option					Each Section of Wall				at Each Section of Wall				
	Floor Area		8d	at 6" O.0			d at 2" C								Type "E"	
	in Square Feet	×	Wall Length	Tie- down	New Fdn Req'd ?	Wall Length	Tie- down	New Fdn Req'd ?	Type "A"	Type "B"	Type "C"	1/2"ø Bolt	5/8"ø Bolt	Type "D"	or "F"	Type "G"
Ē	800		3'-6"	TD1	No	2'-8"	TD2	Yes	1	2	2	2	1	3	3	3
Light Construction	1000		4'-0"	TD1	No	2'-8"	TD3	Yes	1	2	3	2	2	3	3	4
	1200		4'-6"	TD1	No	2'-8"	TD4	Yes	2	2	3	3	2	4	4	5
	1500		6'-0"	TD1	No	3'-0"	TD4	Yes	2	3	4	3	2	5	4	6
	2000		8'-0"	TD1	No	3'-6"	TD4	Yes	2	4	5	4	3	6	6	7
tion	800		4'-0"	TD2	Yes	2'-8'	TD4	Yes	2	2	3	2	2	3	3	4
Construction	1000		5'-0"	TD1	No	2'-8"	TD4	Yes	2	3	3	3	2	4	4	5
Con	1200		6'-0"	TD1	No	3'-0"	TD4	Yes	2	3	4	3	2	5	5	6
Medium	1500		7'-6"	TD1	No	3'-6"	TD5	Yes	2	4	5	4	3	6	6	7
Me	2000		10'-0"	TD1	No	4'-0"	TD5	Yes	3	5	6	5	4	7	7	9
Heavy Construction	800		5'-6"	TD1	No	2'-8"	TD4	Yes	2	3	3	3	2	4	4	5
	1000		6'-6"	TD1	No	3'-0"	TD4	Yes	2	3	4	3	2	5	5	6
	1200		8'-0"	TD1	No	3'-6"	TD4	Yes	3	4	5	4	3	6	6	7
	1500		10'-0"	None	No	4'-0"	TD4	Yes	3	5	6	5	3	7	7	9
He	2000		13'-0"	None	No	5'-6"	TD4	Yes	4	6	8	6	4	10	9	12

Table in Figure 5.4-11 – Earthquake Retrofit Schedule at $S_{DS} = 1.0$ at front of garage in dwelling with a ground story residential unit with two sections of wood structural panel shear wall.

EARTHQUAKE RETROFIT SCHEDULE (S _{DS} = 1.0)								
① NOIL	2	applies ©		IMN RETROFIT) (3)	Tensor Proprietary Shear Wall Retrofit (4) (5)	© DIAPHRAGM NAILING (3) (5)		
WEIGHT CLASSIFICATION	Floor Area in Square Feet		Steel Column	Column Connection Type (per detail 2 on sheet D7) at Upper Floor	Minimum Required at Allowable Shear Capacity (Pounds)	Edge Nail Spacing		
c c	800		W8x21	C1	3250	6		
Light Construction	1000		W8x21	C1	4060	6		
onstr	1200		W8x28	C1	4870	6		
jt C	1500		W8x28	C1	6090	6		
ij	2000		W10x30	C2	8120	6		
Medium Construction	800		W8x21	C1	4200	6		
	1000		W8x28	C1	5250	6		
	1200		W8x28	C1	6300	6		
	1500		W10x30	C2	7880	6		
	2000		W12x35	C3	10,500	6		
Heavy Construction	800		W8x28	C1	5400	6		
	1000		W8x28	C1	6860	4		
	1200		W10x30	C2	8230	4		
	1500		W12x35	C3	10,300	4		
Η̈́	2000		W10x45	C3	13,700	4		

Table in Figure 5.4-8 – Alternate Earthquake Retrofit Schedule at $S_{\rm DS}$ = 1.0 with steel column or proprietary shear wall.

EARTHQUAKE RETROFIT SCHEDULE (S _{DS} = 1.2)								
T ATION (C)	2	applies ©		IMN RETROFIT) (3)	The state of the s	© DIAPHRAGM NAILING (3) (5)		
WEIGHT CLASSSIFICATION	Floor Area in Square Feet	☑ Mark row that applies	Steel Column	Column Connection Type (per detail 2 on sheet D7) at Upper Floor	Minimum Required at Allowable Shear Capacity (lbs)	Edge Nail Spacing		
Ē	800		W8x21	C1	3900	6		
Light Construction	1000		W8x28	C1	4870	6		
onsti	1200		W8x28	C1	5850	6		
ght C	1500		W10x26	C2	7310	6		
Li	2000		W12x35	C2	9740	6		
tion	800		W8x28	C1	5040	6		
struci	1000		W8x28	C1	6300	6		
Cons	1200		W10x30	C2	7560	4		
Medium Construction	1500		W12x35	C2	9450	4		
Мес	2000		W10x45	C3	12,600	4		
E C	800		W8x28	C1	6590	4		
ructic	1000		W10x30	C2	8230	4		
onstr	1200		W12x35	C2	9880	3		
Heavy Construction	1500		W12x35	C3	12,400	3		
Нев	2000		"NG"	"NG"	16,500	3		

Table in Figure 5.4-15 – Alternate Earthquake Retrofit Schedule at $S_{DS} = 1.2$ with steel column or proprietary shear wall.