

# The ATC 78 Methodology for Evaluation and Mitigation of Non-Ductile Concrete Buildings

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**University of Colorado, Boulder**



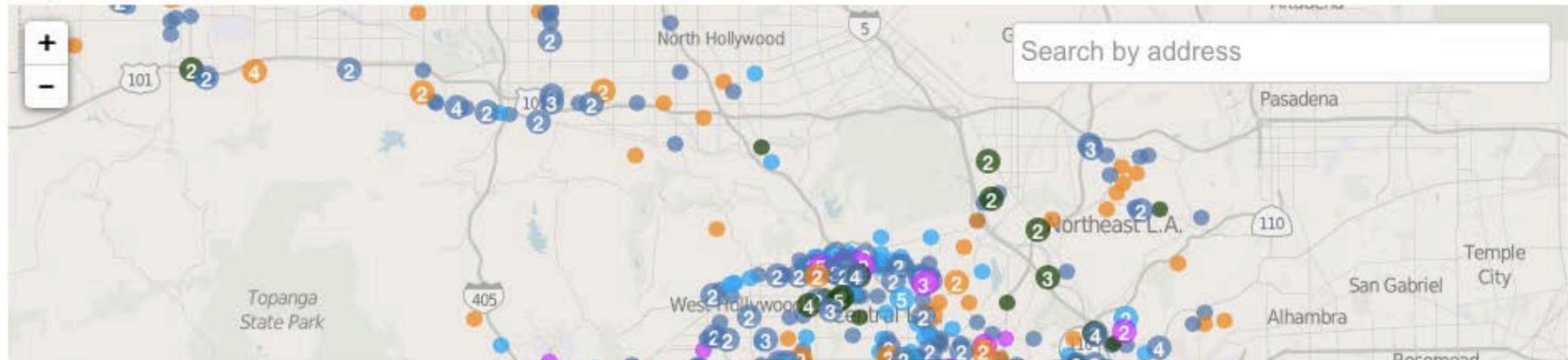
**FEMA**

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ALL
RESIDENTIAL
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EDUCATION
RELIGIOUS



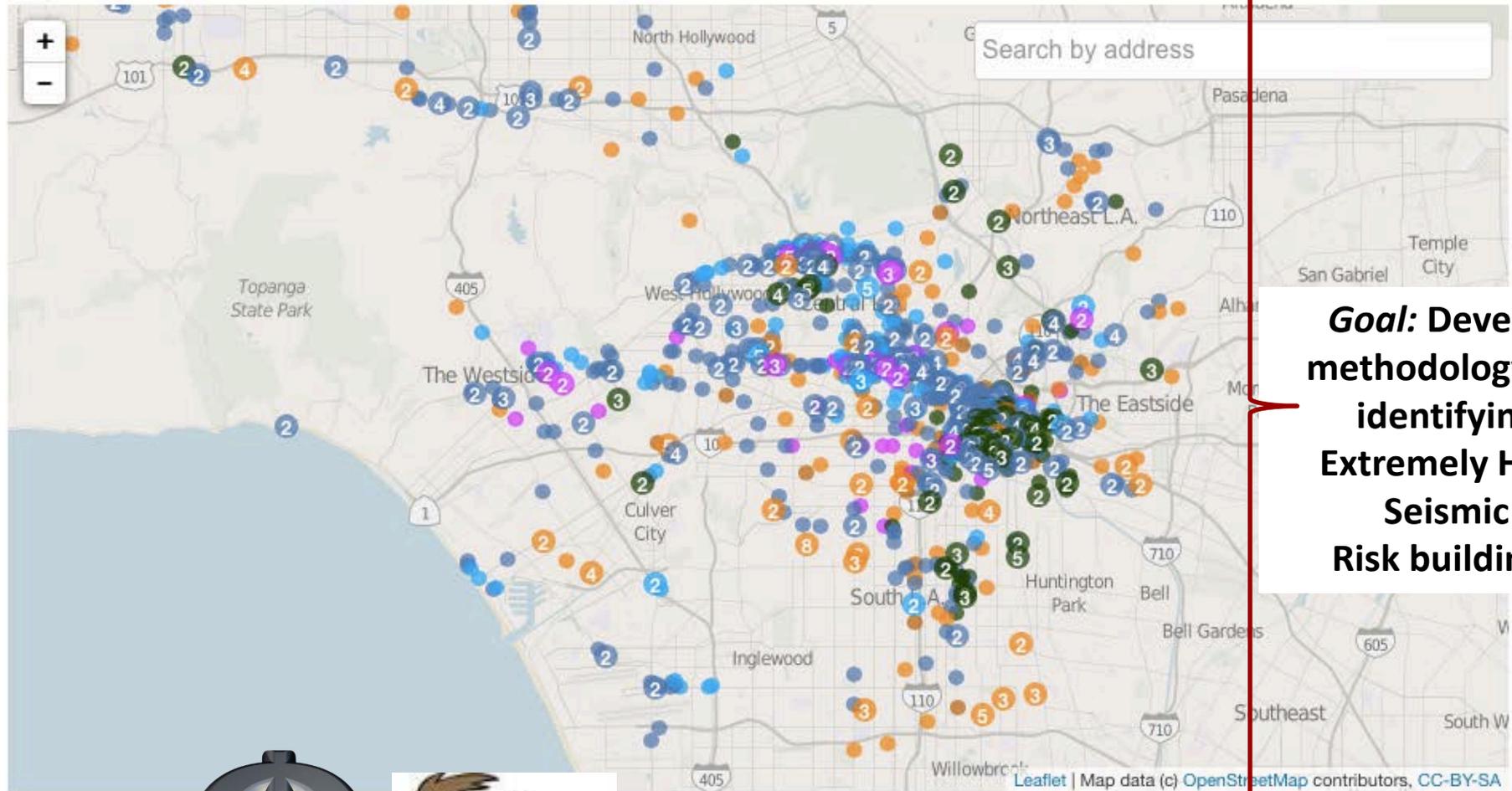
11980 SAN VICENTE BLVD		Parking structure	4	1975	80,298	2,792,764	16	0
11999 CHALON RD	Mt St Marys College Chalon Campus	School	4		100,000	11,468,000	0	0
1200 WILSHIRE BLVD		Office	6	1970	95,075	9,408,622	0	0
1201 E WASHINGTON BLVD		Commercial	2	1936	39,679	2,245,038	40	0
121 E 9TH ST		Office	12	1926	274,480	24,211,881	8	0
122 E 7TH ST		Industrial	13	1926	116,896	9,189,195	4	35
1225 E 52ND ST	Hooper Avenue	School	2		19,270	1,738,539	385	0
12326 RIVERSIDE DR		School	2	1964	13,695	1,235,563	274	0
1248 PALMETTO ST		Industrial	1	1970	102,998	7,603,312	5	31
12500 BRADDOCK DR	Marina Del Rey MS	School	1		1,500	18,500	60	0
12540 PIERCE AVE	Maclay, Chas	School	1		26,000	2,345,720	0	0
126 E BLOOM ST	Ann Street	School	1		13,953	1,258,840	279	0



reetMap contributors, CC-BY-SA

## Los Angeles Times, Older Concrete Buildings

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**Goal: Develop methodology for identifying Extremely High Seismic Risk buildings**



**Cost-Effective**



**Simple  
Repeatable**

**Avoid excessive conservatism**

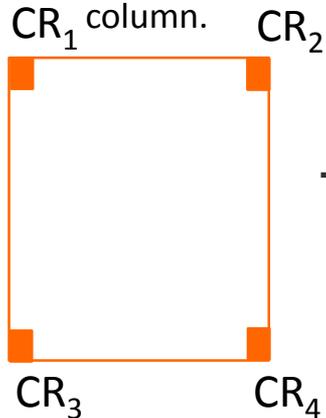
# Guiding Principles

- Focused on collapse, taken as loss of gravity support in a story, considering
  - Weak stories
  - Torsion
  - Axial load drift demand and shear capacity of column
  - Punching shear failure of slab-column connections
- Collapse risk evaluated through estimation of median drift demands and capacities, determined without needing a nonlinear model

# Overview

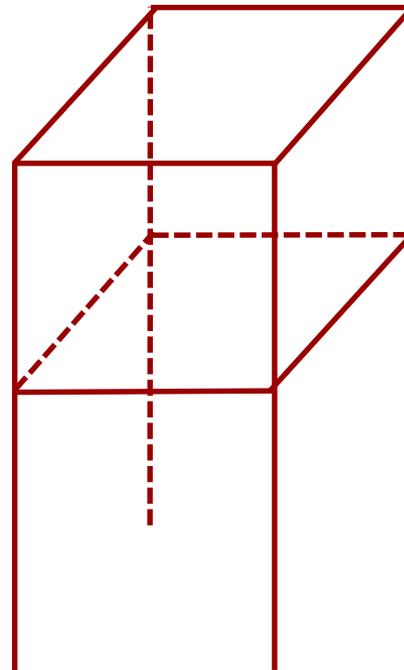
## Column Ratings

Column ratings are a function of the drift demand and drift capacity on each  $CR_1$  column.



→ Story rating depends on ratings of individual column and adjacency of poor columns

## Story Ratings

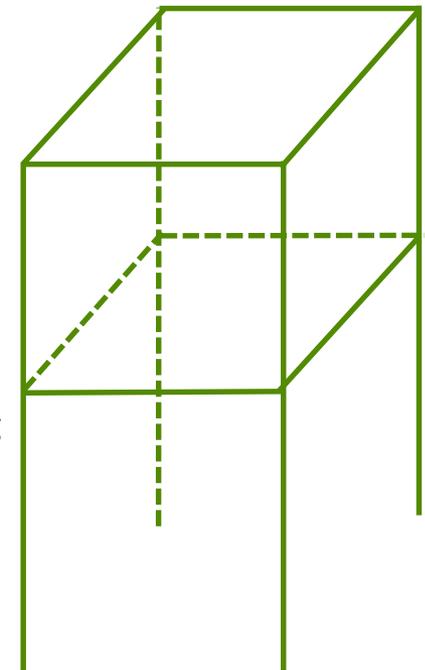


$SR_2$

$SR_1$

→ Building rating corresponds to worst story rating

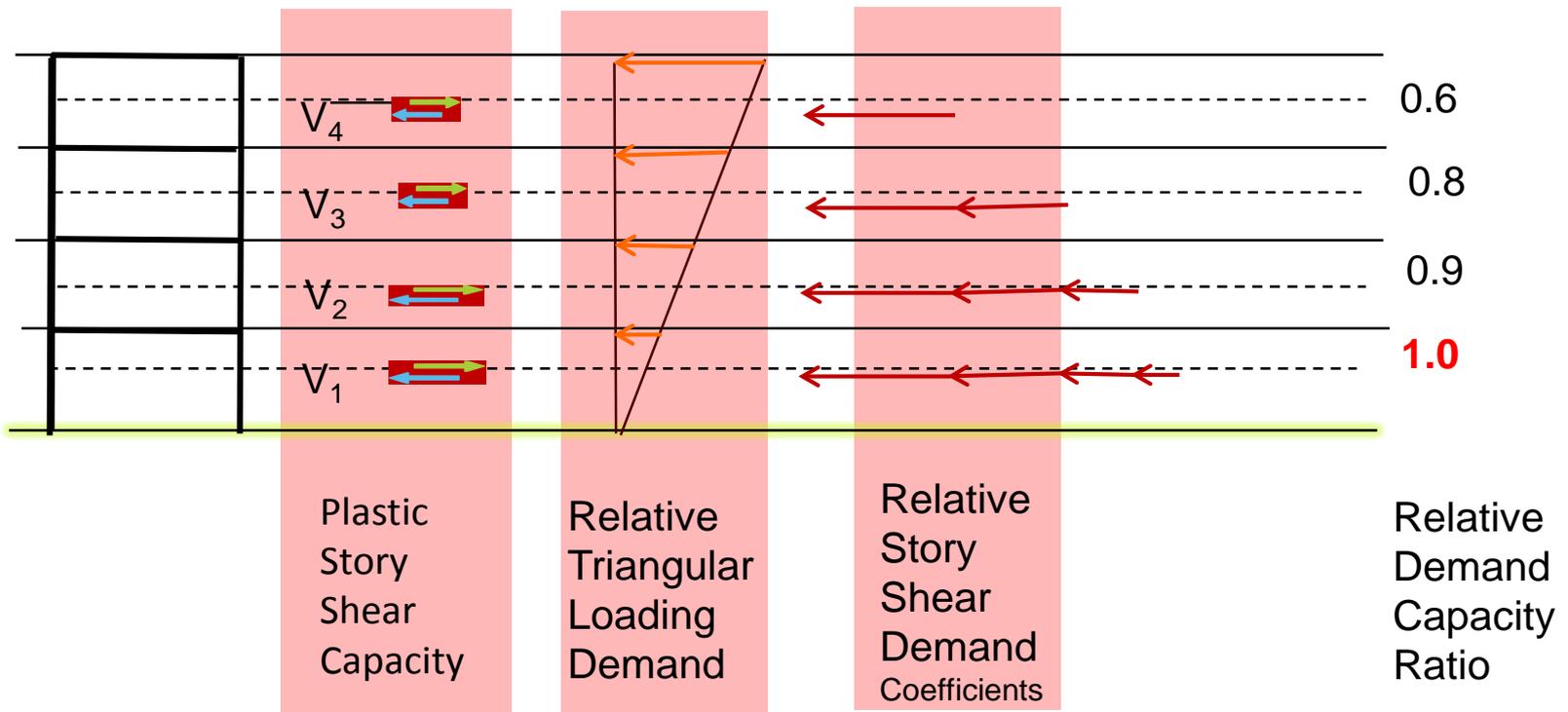
## Building Rating



All ratings range from 0 to 1, where a value of 1 indicates a high likelihood of failure for the level of excitation considered.

# Story Demand Capacity Ratios

“Simulate” pushover to get story demands and capacities



# Period Estimation

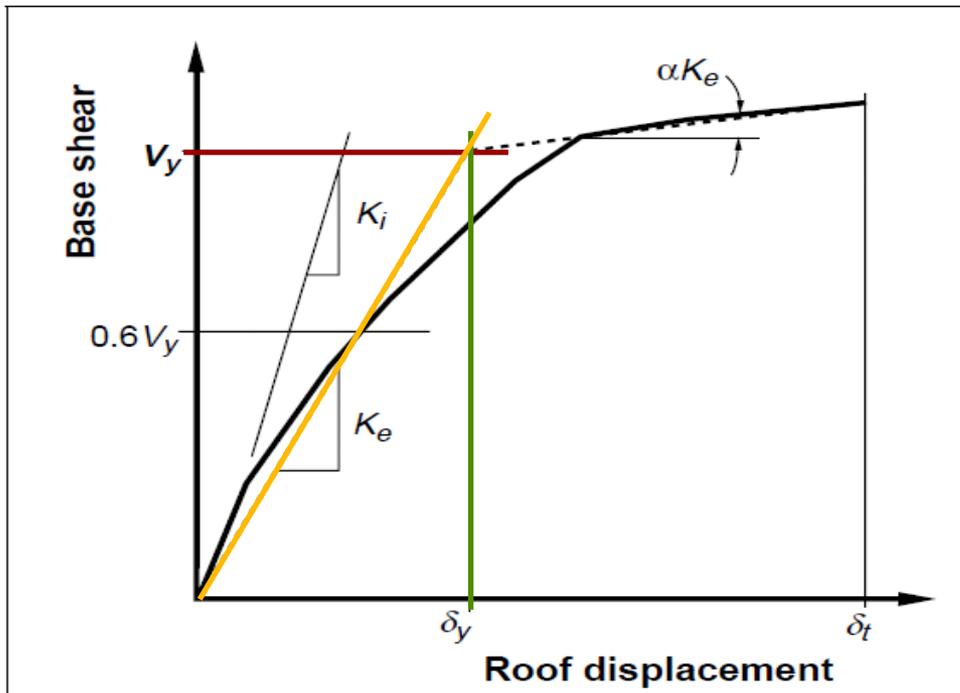
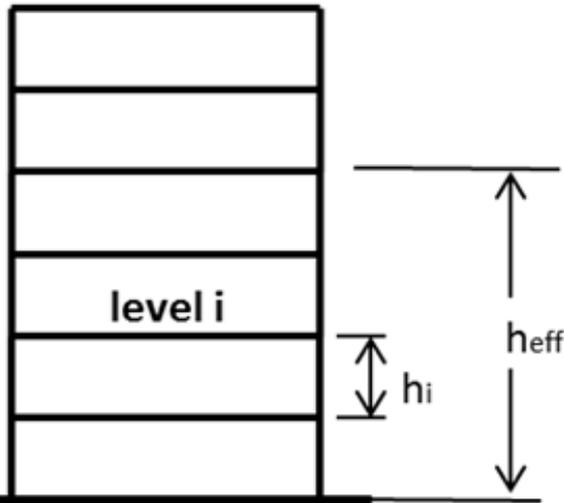


Figure 3-1 Calculation of Effective Stiffness,  $K_e$

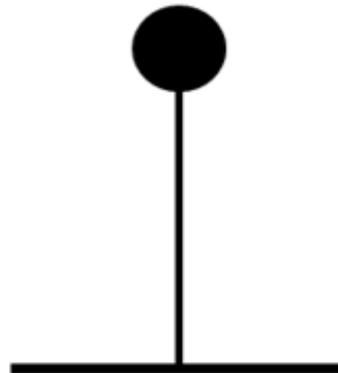
$V_y$  from plastic story shear capacities

$\delta_y$  from assumed structural yield drift

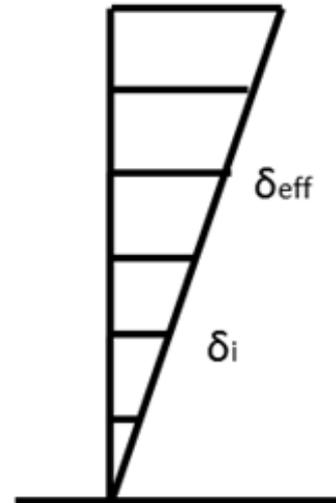
# Story Drift Demands



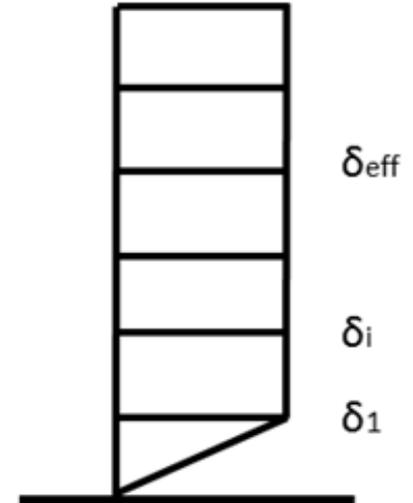
(a) Building Elevation



(b) Equivalent SDOF system



(c) Idealized uniform drift pattern



(d) Idealized weak story drift pattern

$$\delta_i = \alpha_i \beta_{ci} \beta_{si} l_{ui} \left( \frac{\delta_{eff}}{h_{eff}} \right) \leq \delta_{eff}$$

# Column Drift Demands

Story drift demands are converted to column drift demands based on two factors

- Torsional amplification of drifts due to inherent and accidental torsion
- Separation of story drifts taken by columns and beams

# Column Drift Capacity

Drift capacity represents the drift at which the column of interest will fail axially

Drift capacity computed from empirical relationships depending on reinforcement and axial load

To quantify drift capacity, need to

- Classify column based on failure mode
- Estimate plastic rotation corresponding to axial failure
- Convert plastic rotation to drift

# Column Drift Capacity

## Column classifications based on failure mode

Ratio of flexural to shear strengths, $V_{pMf}/V_{nj}^{**}$	Transverse Reinforcement Detail		
	ACI-conforming details with 135° hooks	Closed hoops with 90° hooks	Lap-spliced or any other reinforcement
$(V_{pMf}/V_{nj}) < 0.6$	<i>i</i> ***	<i>ii</i>	<i>ii</i>
$0.6 \leq (V_{pMf}/V_{nj}) \leq 1.1$	<i>ii</i>	<i>ii</i>	<i>iii</i>
$(V_{pMf}/V_{nj}) > 1.1$	<i>iii</i>	<i>iii</i>	<i>iii</i>

\* Adopted from Li et al., 2014

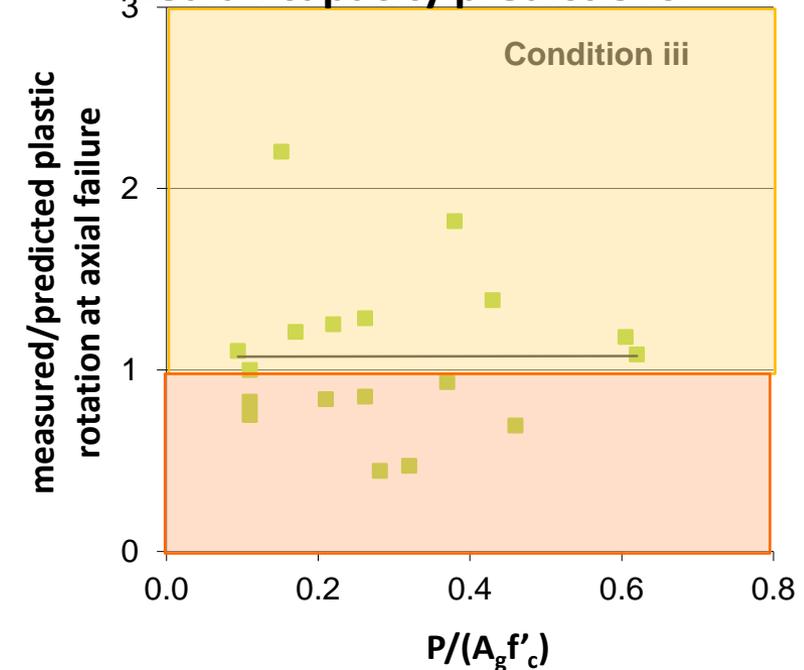
# Column Drift Capacity

Column plastic rotation capacity representing axial capacity

Condition	Axial load ratio, $P/A_g f'_c$	Shear reinforcement ratio, $A_v/b_w s$	Plastic Rotation Capacity, $\theta_c$
i	$\leq 0.1$	$\geq 0.006$	0.090
	$\geq 0.6$	$\geq 0.006$	0.030
	$\leq 0.1$	$= 0.002$	0.050
	$\geq 0.6$	$= 0.002$	0.018
ii	$\leq 0.1$	$\geq 0.006$	0.082
	$\geq 0.6$	$\geq 0.006$	0.023
	$\leq 0.1$	$\leq 0.0005$	0.025
	$\geq 0.6$	$\leq 0.0005$	0.011
iii	$\leq 0.1$	$\geq 0.006$	0.075
	$\geq 0.6$	$\geq 0.006$	0.020
	$\leq 0.1$	$\leq 0.0005$	0.016
	$\geq 0.6$	$\leq 0.0005$	0.006

\* For axial load and shear reinforcement ratios between the tabulated values, calculate the plastic rotation capacity via linear interpolation.

Tabulated values developed by ATC-78 team from empirical data. Represent median capacity predictions.



# Column Rating

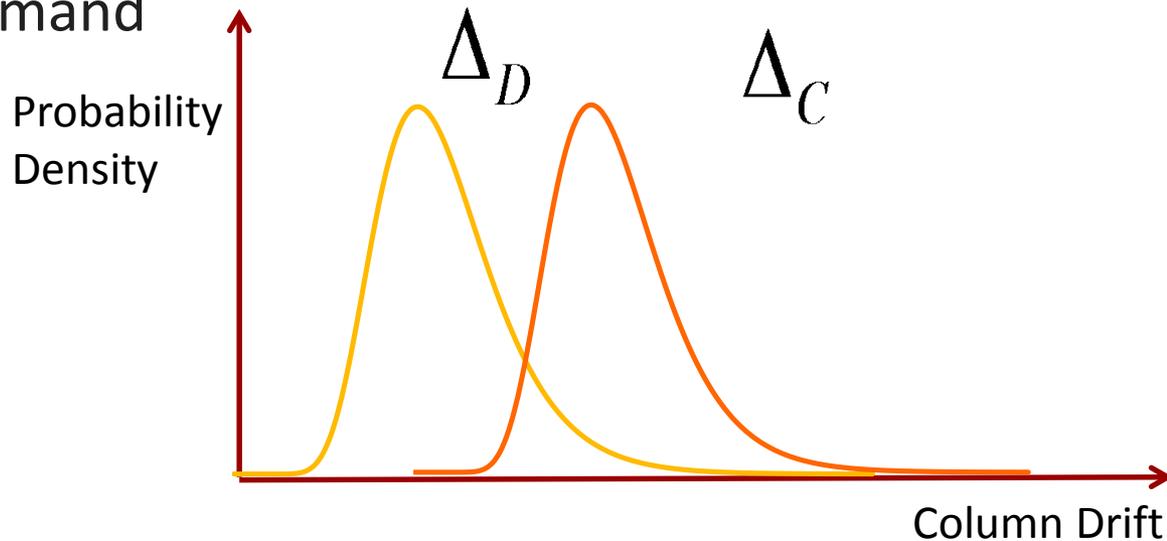
Column rating represents the probability that the drift demand exceeds the drift capacity.

Computed from structural reliability methods where

$$\beta = \frac{\ln(\tilde{\Delta}_C) - \ln(\tilde{\Delta}_D)}{\sqrt{\sigma_{\ln, \Delta_C}^2 + \sigma_{\ln, \Delta_D}^2}}$$

Calculations of drift demands and drift capacities

Uncertainties based on test data and structural analysis results, taken as  $\sigma_{\ln, \Delta_C} = \sigma_{\ln, \Delta_D} = 0.6$



# Column Ratings

Table Used for Determining  
Column Ratings

$\Delta c/\Delta d$	Column Rating, $CR_j$
$\geq 1.75$	0.00
1.5	0.02
1.4	0.04
1.3	0.09
1.2	0.17
1.1	0.31
1.05	0.40
1	0.50
0.95	0.61
0.9	0.71
0.85	0.80
0.8	0.88
0.75	0.93
0.7	0.97
0.65	0.99
$\leq 0.6$	1.00

Higher ratio of capacity vs. demand gives lower CR

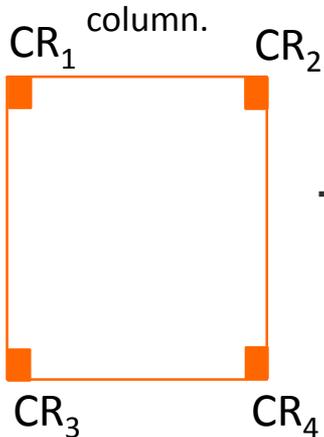
Lower ratio of capacity vs. demand gives higher CR

Other values obtained by linear interpolation

# Story Rating

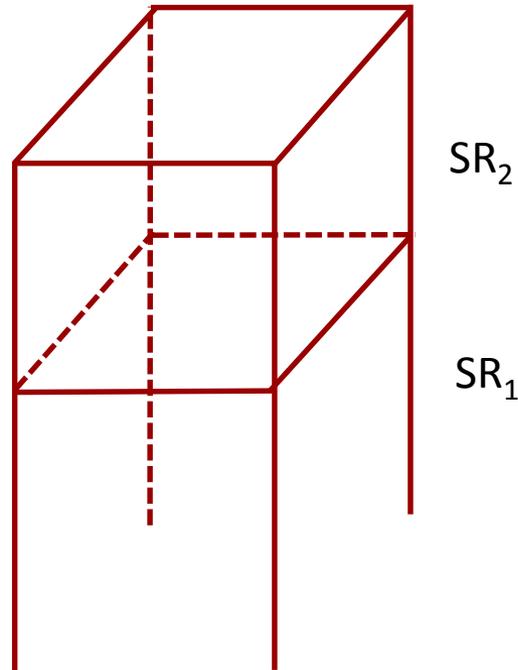
## Column Ratings

Column ratings are a function of the drift demand and drift capacity on each column.



→  
Story rating depends on ratings of individual column and adjacency of poor columns

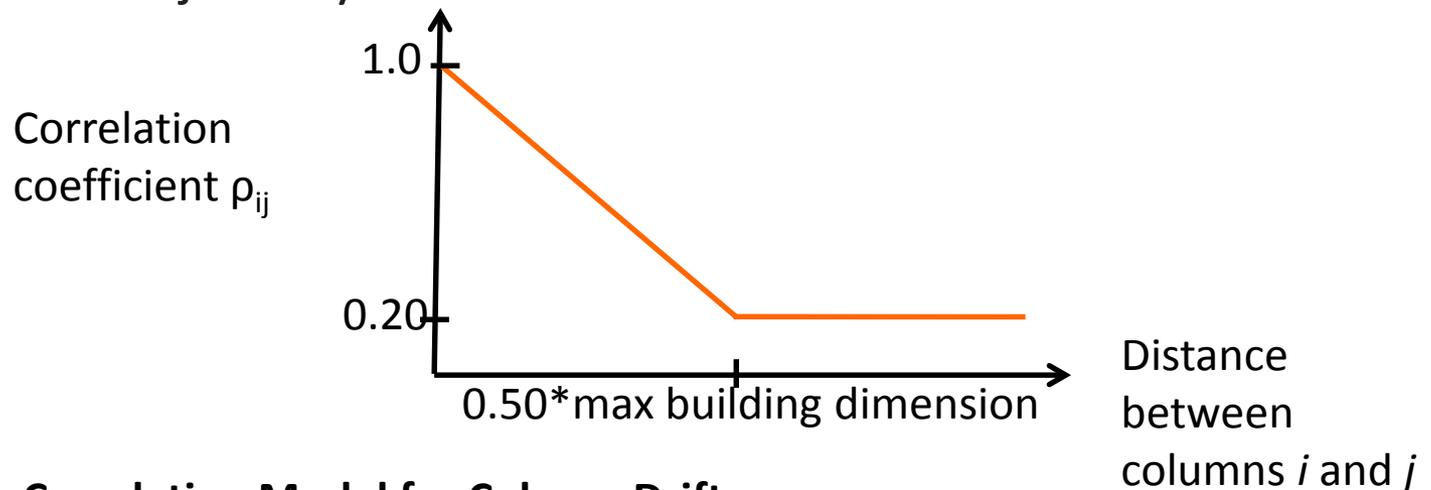
## Story Ratings



# Story Ratings

Story rating represents the probability of story failure

- Story failure occurs if 25% of columns in a story fail
- Column demand is uncertain, but assumed to be perfectly correlated for all columns in a story
- Column capacity is uncertain, and correlations are assumed to be a function of adjacency



**Correlation Model for Column Drift Capacities**

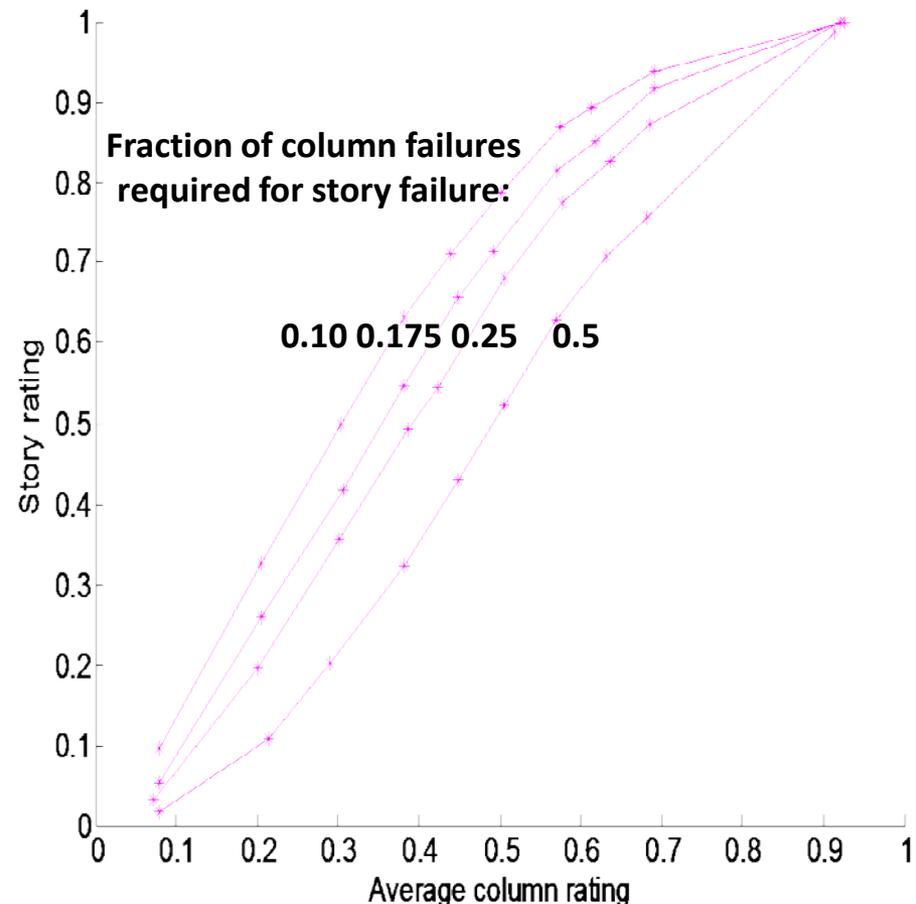
# Story Rating

Adjusted Average Column Rating for Story $i$ , $CR_{i,avg}$	Story Rating for Story $i$ , $SR_i$
$CR_{i,avg} < 0.06$	0.0
$0.06 \leq CR_{i,avg} < 0.16$	0.1
$0.16 \leq CR_{i,avg} < 0.23$	0.2
$0.23 \leq CR_{i,avg} < 0.29$	0.3
$0.29 \leq CR_{i,avg} < 0.36$	0.4
$0.36 \leq CR_{i,avg} < 0.42$	0.5
$0.42 \leq CR_{i,avg} < 0.50$	0.6
$0.50 \leq CR_{i,avg} < 0.58$	0.7
$0.58 \leq CR_{i,avg} < 0.68$	0.8
$0.68 \leq CR_{i,avg} < 0.87$	0.9
$\geq 0.87$	1.0

# Story Rating

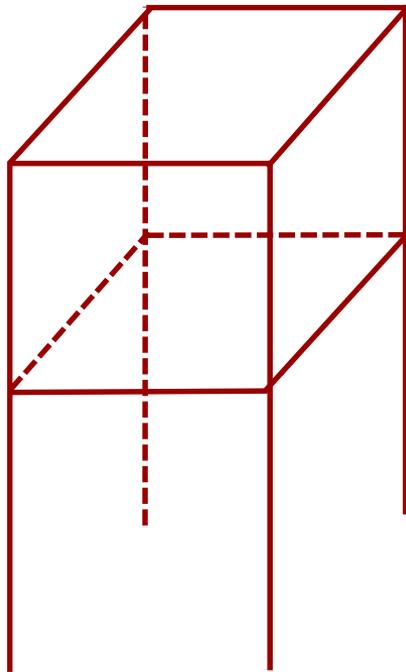
Table development required:

- Monte Carlo simulation: realizations of column demand and capacity were randomly generated
- Simulations accounted for correlation models
- Story failure identified as occurring if more than 25% of the columns failed
- Process repeated to cover range of column ratings



# Building Rating

Story Ratings

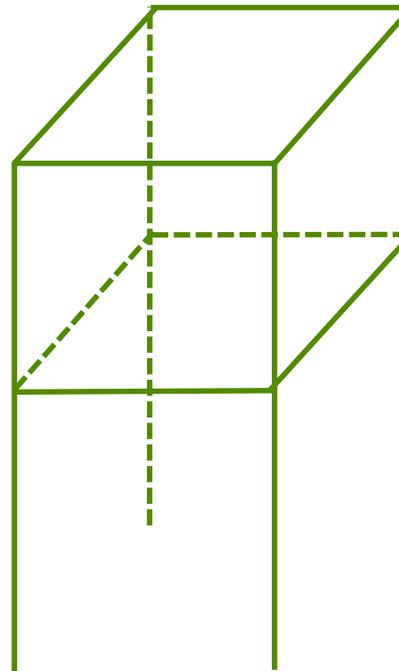


$SR_2$

$SR_1$

Building rating  
corresponds to  
worst story rating

Building Rating



BR

computed in x and y  
directions, where higher  
rating governs

Building ratings  
can be used to  
rank buildings.  
Building rating  
cut-off will be  
used to identify  
*Exceptionally  
High Seismic  
Risk Buildings.*

# Initial Evaluation Efforts

## Student Project at University of Colorado

- 9 Buildings
- 9 Teams
  - 2 Students per team
- Weekly submittals
  - Intermediate calculations were checked

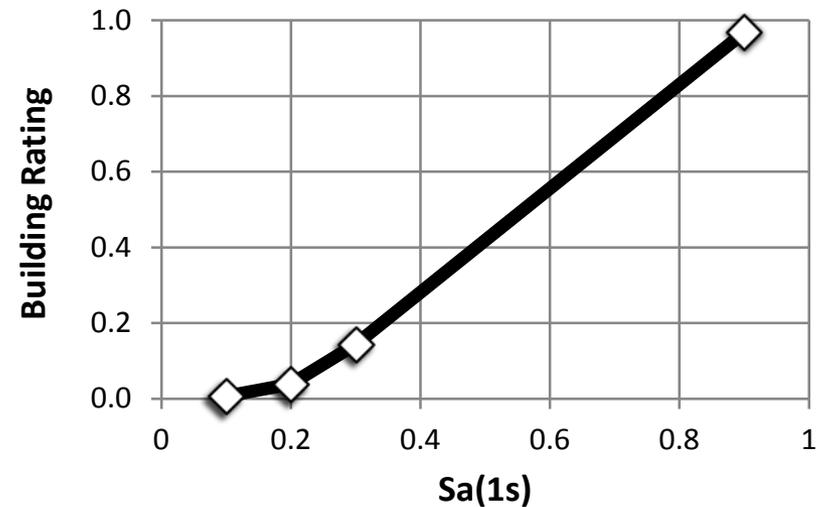
# Initial Evaluation Efforts

Bldg Rating	Number of Buildings with a Given Rating			
	Rating			
	Sa Level 1	Sa Level 2	Sa Level 3	Sa Level 4
< 0.2	9	8	7	2
0.2 - 0.4	-	1	2	1
0.4 - 0.6	-	-	-	-
0.6 - 0.8	-	-	-	2
> 0.8	-	-	-	4

Lowest  
Sa

Highest  
Sa

Fragility Curve for Selected Building



# Next Steps

- Trial evaluation of frame buildings
- Extend procedures to include wall buildings